AE 481W

[Submitted: 10/28/2009]

Matthew Dabrowski Construction Management Consultant: Dr. Chris Magent



[Rydal Park Medical Center Addition] [Rydal, Pennsylvania]

[TECHNICAL ASSIGNMENT 2]

[The following report presents a technical overview of the Rydal Park Medical Center Addition. Located within this assignment an in depth discussion of the addition which has been addressed through the exploration of the detailed project schedule, migration of the site through specific construction phases, analysis of the structural system and associated costs, and finally a brief dialogue regarding the general conditions estimate.]



PROJECT INFORMATION:

FUNCTION: INSTITUTIONAL CARE
BUILDING COST: \$26.590,000

Size: 142,862 Square Feet

DATES OF CONSTRUCTION

SEPT 09' - MARCH 11'

Delivery Method : CM @ Risk, Design-

BID-BUILD W/ NEGOCIATED GMP

PROJECT TEAM:

OWNER: PRESBY'S INSPIRED LIFE

DEVELOPERS: GREENBRIER DEVELOPERS, INC. ARCHITECT: STEWART-CONNERS PLLC

CONSTRUCTION MANAGER:

THE WHITING-TURNER CONTRACTING CO. STRUCTURAL ENGINEER: WK DICKSON & CO.

MEP ENGINEER: MOORE ENGINEERING

ARCHITECTURE:

- AESTHETICS INTENDED TO INVOKE SENSE OF RESIDENTIAL COMMUNITY LIVING AT A LOCATION WHERE SENIORS MAY RECEIVE SKILLED ELDERLY NURSING CARE.
 - 5 STORY STRUCTURE WILL INCLUDE:
 - TWO FLOORS OF PARKING GARAGE SPACE
 - TWO FLOORS OF SKILLED NURSING CARE
 - ONE FLOOR OF CRITICAL MEMORY SUPPORT
- FAÇADE WILL IMPLEMENT A STONE VENEER SYSTEM AND SPRAY APPLIED STUCCO AS WELL AS CURTAIN WINDOW WALL & PELLA WINDOWS TO MATCH THE EXISTING MEDICAL FACILITY

STRUCTURAL:

FOUNDATION:

 Helical Geo-Pier Stone Column Foundation System Will Provide Support under Spread Footers

SUPERSTRUCTURE:

POST-TENSION TWO-WAY CONCRETE SYSTEM
 REINFORCED CONCRETE COLUMNS

 Reinforced masonry mass shear Walls (gravity system), located mainly at stairtowers, utilized as the lateral system

ROOF STURCUTRE:

 Non-composite Roof Deck Mainly Supported by K-Series Joists and Severl Intermediate wide flange Beams Between Columns

MEP SYSTEMS:

- FOUR PIPE AIR/WATER HVAC SYSTEM:
 - THREE FAN COIL UNITS (400 1200 CFM)
 - EIGHT AHU'S (630 3770 CFM)
 - FOUR ENERGY RECOVERY UNITS (FIRST FLOOR)
- BUILDING POWER SUPPLIED BY PECO:
 - 15 KW SWITCHGEAR TO STEPDOWN POWER - 208/120V 3 Phase 4 wire wire System
 - 350 KW EMERGENCY GENERATOR (FIRST FLOOR)
- COMBINATION DRY AND WET PIPE FIRE SUPRESSION SYSTEM





MATTHEW JAMES DABROWSKI
ARCHITECTURAL ENGINEERING | CONSTRUCTION MANAGEMENT http://www.engr.psu.edu/ae/thesis/portfolios/2010/mjd5060

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Executive Summary

[Submitted: 10/28/2009]

Presby's Inspired Life develops and manages continuing care communities that provide an opportunity for senior citizens to live their lives within a relaxing residential surrounding while retaining peace of mind that if any health emergency were to arise, assistance would be immediately available. This location, Rydal Park, is a continuing care retirement community where seniors begin living at homes that are cozy cottages and as their conditions progress (if any exist), they will eventually move into the medical facility at the center of the campus. This medical addition has finishes that closely resemble would be found within a luxury hotel, but with the added necessity of being equipped for medical emergencies.

Technical Assignment 2 digs deeper into specific portions of the Medical Center Addition. The first part of the research will focus on detailed planning through the development of a detailed schedule and the creation of several phase specific site plans. Along with the site plans, a construction sequence diagram will illustrate the flow and path of work throughout the construction phase. Following this detailed planning portion, an in depth cost analysis of the concrete structural system and general conditions will be addressed. The assignment will conclude with a summary of the 18th Annual PACE conference examining major current industry issues and which pertain to this project.

The project schedule, developed for the first technical assignment, was expanded upon to gain an improved understanding of the building sequence. A total of 143 activities have been developed for this five story structure. Notice to proceed was granted to the Whiting-Turner Contracting Co. on October 21st, 2009. Substantial completion must occur by the end of August 2011 to meet the contractual agreements signed with the owner.

Following the detailed planning portion, an analysis of the post tension concrete structural system as well as the general conditions estimate will examine all associated costs. The structural system analysis involves the footers, grade beams, SOG's, columns, shear walls, and elevated slabs which resulted with an estimated value of \$3,722,270. This value is approximately 18% below the actual submitted lump sum value received from the subcontractor. Reasons for this low estimate will be discussed at length due to the fact that this percentage of error is relatively high in comparison to the level of detail performed for the take off.

Upon completion of the general conditions estimate, the final costs came to \$2,712,245, with the weekly costs at \$34,772.37 for the 18 month duration of the project. This value is about 10% of the total project cost which appears to be within a reasonable cost range.

Detailed Project Schedule

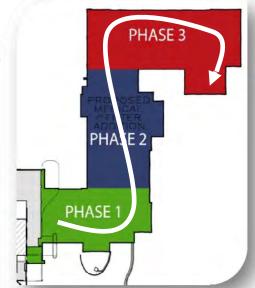
Stewart-Conners Architects, who also designs many of Presby's Inpsired Life's retirement communities, began designing this Medical Center Addition in February and March of 2008. As the schematic design phase started wrapping up, The Whiting-Turner Contracting Co. was hired in June 2008 for full preconstruction and construction management services. Due to the private nature of this project, the construction management services (at risk) were not put out for public bidding. From June 2008 to July 2009 it was unsure whether or not this project would see life as it went on hold twice, each time for a couple months. During July 2009, Whiting-Turner began increasing its effort to improve the success of the project eventually leading to the acceptance of a negotiated GMP on October 21st, 2009. Notice to proceed was given on this date and Whiting-Turner plans to be completely mobilized no later than mid-November '09. Currently, Whiting-Turner has bought-out site work, GeoPier foundations, concrete and MEP contractors. Achieving a watertight building is planned for the end of November '10 and substantial completion by the end of July '11. Meeting this deadline is crucial due to occupancy phasing requirements that have been dictated by the owner.

In order to properly interpret the detailed project schedule, several key features must be addressed. The construction phase of the schedule has been broken down into four major portions; substructure/subgrade, concrete structure, building enclosure, and interiors. In an attempt to remove confusion with relationships, most but not all, activities have been linked in a finish-start fashion. Within each major portion of construction, the work has been broken down by floors. It is also important to note that this 143 activity schedule was extrapolated from a basic 25 activity schedule. This presented challenges when developing appropriate durations for specific activities. Some items may look out of place, but there has been thought and reason behind each item's time location. Electrical equipment within the interiors phase is shown as being set during the structure phase. Since most mechanical and electrical equipment is bulky, large and expensive, it must be set while the floor above is open and easily accessible. Finally, one key feature to note regarding this schedule is that the MEP trades will be on site for the duration of the project. Between utility relocation, utility feeds and service, major equipment placement, MEP rough-in, and fixtures, the electrical and mechanical subcontractors will be performing work during each major construction phase.

Project Sequencing

[Submitted: 10/28/2009]

In order to properly sequence this project, this construction flow diagramed was developed. This flow was originally created for the concrete pour schedule, but was later decided that it also provides clear direction and general breakdown for each floor. Phase 1 was located because of its proximity to the existing medical facility. Once Phase 1 is completed on each floor, the rooms in the existing facility which have been closed due to the construction can be utilized once again. The owner has placed less emphasis on phases 2 and 3 since they don't impose the same issue or threat on the existing facility. Utilizing a three phased breakdown per floor will allow for high detailing when scheduling the concrete and building envelope subcontractors. Within each one of the four major construction phases, the schedule is broken down by floor. Breaking down the schedule into these four major



[Figure 01. Construction Flow]

construction phases will allow Whiting-Turner to improve their understanding of each activity through keeping related tasks grouped.

The Detailed Project Schedule is located within **Appendix A.**

Site Layout Plan

Due to the "addition" nature of this project, the building footprint is sitting on a site that was not originally designed or intended to accept a building. The site is currently a parking lot which is bounded by a grove of trees, Rydal Road, and The Fairway to the east. Along the south and west sides of the site are parking lots and existing buildings which may not be impacted by the construction in any form. Located at the north end of the site is a parking lot which will house parking for authorized construction personnel and the site trailers. For the safety of the community residents, the owner has decided to close all pedestrian walkways within the

tree groves. The only nearby walkway that is to remain open is the sidewalk along The Fairway and Rydal road. The only period that this walkway will be an issue is during the tree clearing phase. Since this is а retirement community, strict working hours of 7-5pm must be adhered to as to not disturb the patients in the medical facility or residents living in the apartments. For the duration of construction, overflow parking for authorized community residents will be relocated to the Whole Foods Lot,

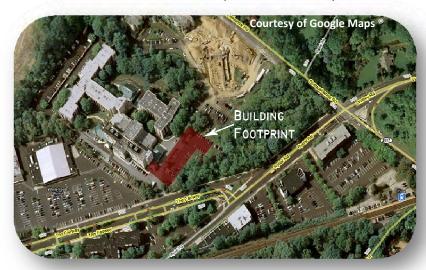


Figure 02. Aerial View of the Site

located directly in front of the south face of the existing medical facility.

General Conditions and Temporary Facilities

Most of the general conditions components that have been located on site will stay within in the same general region for the duration of the project. The delivery gate has been located at the south end of the site which will allow for quick and easy access to the loading dock and man hoist. Two regions have been selected for dumpsters, both of which are located with road access for ease of dumpster pickup. The main construction parking lot will provide space for a maximum of four contractor trailers. Trailers on site must be properly coordinated to ensure that each contractor has the required space to manage their specific work. If more space is required, interior-focused subcontractors will be allowed to set up within the completed parking garage space on the ground level. Toilets have been located within the parking lot, within close proximity to the site trailers. A temporary power shed, located at the southeast corner of the construction parking lot, will power the trailers.

Excavation Phase: Site Plan Summary

[Submitted: 10/28/2009]

After the geotechnical reports indicated that the site soil mainly consisted of variable fill materials, as well as loose to medium dense residual soils, it was decided to utilize the suggested GeoPier foundations type. As an

added benefit, this foundation system requires a minimal amount of soil removal minimizing site excavation. Due to the fact that there is relatively no space on this campus to store excavated soil, this foundation type was a perfect match for the project needs. The excavation phase of this project will mainly consist of the asphalt parking lot demolition, GeoPier drilling, sediment and erosion control set-up, tree removal, and site grading. The region to the east of the site will provide the required space for the tower crane, soil storage, and as a lay down region after the tree grove has been cleared and graded. During this phase of construction, sediment and erosion controls will be extremely critical to ensure proper storm water pollution management.

Erection Phase: Site Plan Summary

During the first week of February 2010, the tower crane will be delivered and assembled on site and will begin placing concrete by the beginning of March. In order to properly reach each corner of the building, a swing radius of 180 feet is required. During this phase of construction, the newly cleared tree grove will provide a region for formwork lay down, staging, and storage trailers. The delivery gate has been located at an ideal position for the crane to easily fill the concrete bucket while maintaining operator visibility. The delivery gate location also provides an excellent queuing region for concrete trucks, given its location off of a

main road. The crane will begin to wrap up its time on site as the window wall system, stone veneer, and EFIS are installed resulting with a watertight building by the end of November 2010.

Interiors Phase: Site Plan Summary

Once the crane is disassembled and removed, a man / material hoist will be built aiding in the movement of materials throughout the building. This hoist has been located directly next to the loading dock, allowing for quick and easy access to delivered items at the loading dock. Delivery trucks will be able to enter the site through the south entrance, drive through the parking garage, unload any materials, and exit through the north end of the site. This traffic route will eliminate time wasted through redirection and delivery driver confusion. The plan found within the appendix provides a clear description of the flow of work and where storage will be permitted.



[Figure 03. General Conditions and Temporary Facilities Plan]

Detailed Structural System Estimate

[Submitted: 10/28/2009]

Due to the fact that this structure is concrete, it was decided to perform the volume analysis through the study of the structural schedules with assistance from Excel. Since this structure has unique bays with varying slab thicknesses and column dimensions, utilizing the single bay method would contain a level of inconsistency and inaccuracy. The components that were taken off for this portion of the assignment included footers, grade beams, slabs on grade, columns, elevated slabs, elevated beams, shear walls, and stairwells. All five levels of this building are above ground, therefore no foundation or subgrade walls are found within this analysis. Basic measurements and quantity take offs were transferred into Excel spreadsheets and applied to the appropriate building component. These spreadsheets immediately calculated the desired cubic yards, tonnages, and formwork contact area for each major structural element. Without the use of an electronic spreadsheet tool, this estimate would not have been a viable option and would have required an extensive amount of work to complete.

Cavan Concrete® was the selected subcontractor for this project, winning the bid with a submitted lump sum value of \$4.61 million (this amount has been slightly rounded per request of WT and Cavan). This value equates to roughly \$28.97 per square foot. A total of eight contractors submitted bids for this contract with a price range varying from \$4.4 million to \$7.2 million.

The following table summarizes the quantities of concrete, rebar, formwork required to build this concrete structure.

Struc	tural Compone	nt Summary	
Item Description	Concrete (CY)	Rebar (Tons)	Formwork (SFCA)
Footing	1408	59	6,431
Slab on Grade	566	N/A	638
Structural PT Slab	3525	159	134,457
Column	337	41	25,619
Grade Beam	143	5	2,142
Beam	63	4	3,018
Shear Walls / Stair Towers	672	60	15,280
	6714 CY	328 Tons	187,585 Tons

[Table 01. Structural System Quantity Take-Off Summary]

After applying these quantities to RS Mean's 2009 data, an estimated value of \$3,774,382 (\$23.72 per sf) was derived. The table found on the next page provides a complete summary of this calculated estimate. Comparing this value to the actual lump sum value reveals that this analysis has a percentage of error of 18%. One potential source of this error could be Cavan's knowledge of pricing post-tension projects. From this estimates point of view, many of the components were priced as just cast-in-place. Extra labor associated with constructing a post tension structure would be captured due to Cavan's specialty insight and knowledge.

Another factor that could have resulted in high bids from contractors to Whiting-Turner was that the architect and structural released two significant drawing addenda during the bid period. Due to this, many contractors may have felt uneasy and unsure as to what new features would be added in future addenda. Contractors may have utilized higher contingencies in order to properly protect themselves.

Finally, it was felt that the discovered ratio of tons of steel to cubic yards of concrete was relatively low. Averaged throughout the entire building, this ratio was found to be approximately 4.88%. The component

that raised the most concern was the structural slab. The post tension slabs had a ratio of 1.98% compared to the columns at 12.2%. Due to this, when estimating the total tonnage of steel within the slabs, a ratio of 4% was utilized in an effort to account for any reinforcing that was possibly missed during multiple slab takeoffs.

The following table summarizes the estimated cost for the materials, labor, and equipment required to erect this structure. RS Means Cost Works 2009 data was utilized for this estimate. These cost units have been adjusted for Philadelphia. Ten percent waste factors where applied to the concrete and rebar value, and fifteen percent was applied to formwork. Waste factors have only been applied to material pricing only, labor or equipment has not been altered.

	Deta	iled Es	tima	ite for Cost	of t	he Post Te	nsic	on Concrete Sy	stei	n				
Item Description	Quantity	Unit	Bar	e Material	Ba	are Labor	Baı	e Equipment	9,	Subtotal	Т	otal O & P	Cal	culated O & P
					Cor	ncrete								
Spread Footings (3000 psi)	1,408	CY	\$	111.10	\$	33.86	\$	12.15	\$	157.11	\$	204.24	\$	287,582.81
Grade Beams (5000 psi)	143	CY	\$	122.10	\$	12.41	\$	4.56	\$	139.07	\$	180.78	\$	25,911.76
Elevated Beams (5000 psi)	63	CY	\$	122.10	\$	36.36	\$	13.15	\$	171.61	\$	223.09	\$	14,004.85
Slab on Grade (4000 psi)	566	CY	\$	116.60	\$	17.36	\$	8.25	\$	142.21	\$	184.87	\$	104,701.21
Columns (5000 psi)	337	CY	\$	122.10	\$	45.36	\$	22.00	\$	189.46	\$	246.30	\$	83,002.43
Shear Walls / Stair Towers (5000 psi)	672	CY	\$	122.10	\$	27.86	\$	13.75	\$	163.71	\$	212.82	\$	143,068.92
Elevated Structural Slabs (5000 psi)	3,525	CY	\$	122.10	\$	22.86	\$	10.90	\$	155.86	\$	202.62	\$	714,314.53
										Subt	otal	Concrete:	\$	1,372,586.52
					R	ebar								
Spread Footings	59	Tons	\$	1,540.00	\$	395.00	\$	-	\$	1,935.00	\$	2,175.00	\$	128,325.00
Grade Beams / Elevated Beams	10	Tons	\$	1,705.00	\$	890.00	\$	-	\$	2,595.00	\$	3,150.00	\$	31,500.00
Slab on Grade(WWF)	303	CSF	\$	0.55	\$	20.50	\$	-	\$	21.05	\$	31.58	\$	9,562.51
Columns	42	Tons	\$	1,705.00	\$	950.00	\$	-	\$	2,655.00	\$	3,250.00	\$	136,500.00
Shear Walls / Stair Towers	60	Tons	\$	1,622.50	\$	1,340.00	\$	-	\$	2,962.50	\$	3,400.00	\$	204,000.00
Elevated Structural Slabs	159	Tons	\$	1,815.00	\$	490.00	\$	-	\$	2,305.00	\$	2,605.00	\$	414,195.00
										S	ubt	otal Rebar:	\$	924,082.51
					Forr	nwork								
Spread Footings	6,431	SFCA	\$	0.17	\$	0.70	\$	5.35	\$	6.22	\$	9.08	\$	58,426.76
Grade Beams / Elevated Beams	5,161	SFCA	\$	0.14	\$	0.90	\$	4.75	\$	5.79	\$	8.45	\$	43,612.15
Slab on Grade	638	SFCA	\$	0.13	\$	1.35	\$	3.25	\$	4.73	\$	6.90	\$	4,405.35
Columns	25,619	SFCA	\$	0.17	\$	0.79	\$	5.65	\$	6.61	\$	9.65	\$	247,336.64
Shear Walls / Stair Towers	15,280	SFCA	\$	0.14	\$	0.78	\$	4.73	\$	5.65	\$	8.25	\$	126,001.37
Elevated Structural Slabs	134,457	SFCA	\$	0.10	\$	1.55	\$	3.43	\$	5.08	\$	7.42	\$	997,930.16
_										Subto	tal I	Formwork:	\$	1,477,712.42

[Table 02. Detailed Structural Systems Cost Estimate]

[Submitted: 10/28/2009]

Grand Total Estimate: \$ 3,774,381.46

Detailed tables showing how each quantity was developed can be located within Appendix C.

General Conditions Estimate

Whiting-Turner has broken up the general conditions estimate into nine sections: mobilization and temporary field office, small tools and equipment, project management and supervision, travel and lodging, plans / permits / postage, special requirements, testing and inspections, site requirements, and building access. The value submitted within the GMP for Division 01 came to \$2,666,500 or approximately \$31,370.60 in weekly costs. In comparison to the entire GMP, the general conditions equate to approximately 10% of the total project cost. This general conditions estimate was designed for approximately 18-20 months worth of on-site, at risk, construction management services.

One outcome of the negotiated GMP between Whiting-Turner and Presby's Inspired Life was that the owner decided to pick up several general condition items. By doing this, WT was able to slightly reduce their general conditions estimate. Items that a medical setting would usually need during construction such as HEPA-VACS and ventilation machines, Presby's agreed to supply. Other items such a temporary utility services to all trailers, testing & inspections, building permits, and basic office supplies will also all be purchased and managed by the owner.

After further inspection of the general conditions estimate, it was discovered that the majority of the costs come from the project management and supervision staff. This project will require a staff of eight people to run efficiently, which is about 65% of the total general conditions value. Of these eight people, the senior project manager will be billed as part-time, due to his multiple active projects.

General Conditions Estimat	e Va	lue
Description		Value
Mobilization and Temporary Field Office	\$	45,075.00
Small Tools and Equipment	\$	4,150.00
Project Management and Supervision	\$	1,733,800.00
Travel and Lodging	\$	45,050.00
Plans, Permits, and Postage	\$	42,000.00
Special Requirements	\$	47,050.00
Testing and Inspections	\$	12,500.00
Site Requiements	\$	631,870.00
Building Access	\$	105,000.00
Grand Total General Conditions	\$	2,666,495.00

[Table 03. General Conditions Summary]

The entire General Condition Estimate can be found with Appendix D.

[Submitted: 10/28/2009]

PACE Roundtable Summary: "Creating Opportunities"

Currently, the construction industry is facing difficult economic times requiring all of its members to develop innovative approaches to foster a successful come-back. The 18th Annual Partnership for Achieving Construction Excellence meeting has been the perfect environment to develop approaches for these critical problems. During this year's PACE roundtable conference, more than 30 industry representatives and construction management professors met with students to develop key issues facing the construction field. Several discussions were held regarding energy usage in the building industry, BIM execution & planning, and business networking. Each of these topics generated potential research possibilities for CM students.

Conference Format and Topics

[Submitted: 10/28/2009]

During this year's PACE conference, three distinct discussions were held where industry members could explain their views about construction while receiving students input. The first discussion introduced a panel of five industry leaders who shared their thoughts regarding the state of the industry. Considering that many students will be entering the workforce next May, this was extremely helpful and interesting. Most members of this panel agreed that healthcare and government work are the two market sectors that will continue to thrive through this economic period. The next portion of the PACE conference broke out in sessions which set up a problem identification and solution development period. This broke out into three separate rooms focusing on energy, BIM execution & planning, or business networking. Finally, this conference concluded with a panel of students providing their point of view regarding the patterns that have emerged from the current generation's use of blending technology and communication.

Business Networking: Expanding Circles and Creating Opportunities.

Improving the communication process as well as collaborative techniques is essential to this "social industry". The Business Networking: Expanding Circles, and Creating Opportunities breakout session provided a chance for students to listen to experienced industry leaders. Each representative had unique point of views regarding how they establish and maintain their network circles.

Starting off this session was a dialogue regarding the shift in industry delivery methods. Reasons for this shift were agreed upon as resulting with improved benefits for owners through collaborative negotiated GMPs. The conversation shifted to Joint ventures, which presented unexpected outcomes. Business leaders discussed their joint venture experiences and the successes they had with it. It was surprising to hear how companies who compete against each other on a day to day basis where able to set aside differences and work together. Several people suggested joining up with a competitor, especially during this economy, to improve their resumes, expand geographically or enter other market sectors.

Another topic that resonated was the importance of general contractor's ability to get close or pair up with architects and designers. Bevan Mace (Balfour Beatty) sketched a diagram showing that architects need to focus their efforts within the schematic design phase, and leave more of the standard design components to the GC/CM and technical designers. In doing this, energy is not wasted through constant redesigns between the architect and designers. This also lets architects focus on the pure aesthetics and primary architectural appearance of the building, which is where they achieve their primary satisfaction. Also GC's can benefit from meeting new clients and owners through architects and designers.

Finally the concluding portion of this breakout session was focused on integrated project delivery (IPD). Each player involved with building design and construction process needs to learn how to integrate their efforts to improve the quality of the delivered product. Analyzing the means through which a building is delivered is another key component to creating a successful integrated delivery. As this session developed, it became clear that everyone did not have a similar definition for IPD. Someone quoted that integrated project delivery is basically "a successful design-build" project. This statement reflects that design-build projects can exist that are less efficient than design-bid-build given the latter has excellent collaboration and integration. The fact that IPD had such a weak industry definition was the most surprising element of this breakout session. IPD should be a required component of design and construction due to its ability to improve efficiency, cut waste, maintain a constant communication circle and advance the quality of a building.

Finally, one of the best quotes heard during this session was to "chase clients, not projects". This is a phrase that is directly in line with the theme of this breakout. As general contractors harness the notion that construction is a "social industry", keeping a steady flow of business and revenue must be managed through a company's client base, rather than focused project hunting.

The following table is a list of contacts who were met during the PACE activities held on October 14th and 15th. The dinner and lunch periods fostered a great environment to relax and converse about general construction matters. A great deal of feedback was given regarding thesis research and assignments.

Contacts Name	Company	Email Address
Michael G. Pittsman	James G. Davis Construction Corp.	mpittsman@davisconstruction.com
James Salvino	Clark Construction Group	james.salvino@clarkconstructon.com
Michael Barnhart	Forrester Construction Company	mbarnhart@forresterconstruction.com
Bevan Mace	Balfour Beatty Construction	bmace@balfourbeattyus.com
Charles Tomasco	Truland Systems Corporation	ctomasco@truland.com

Thesis Relevant Topics

[Submitted: 10/28/2009]

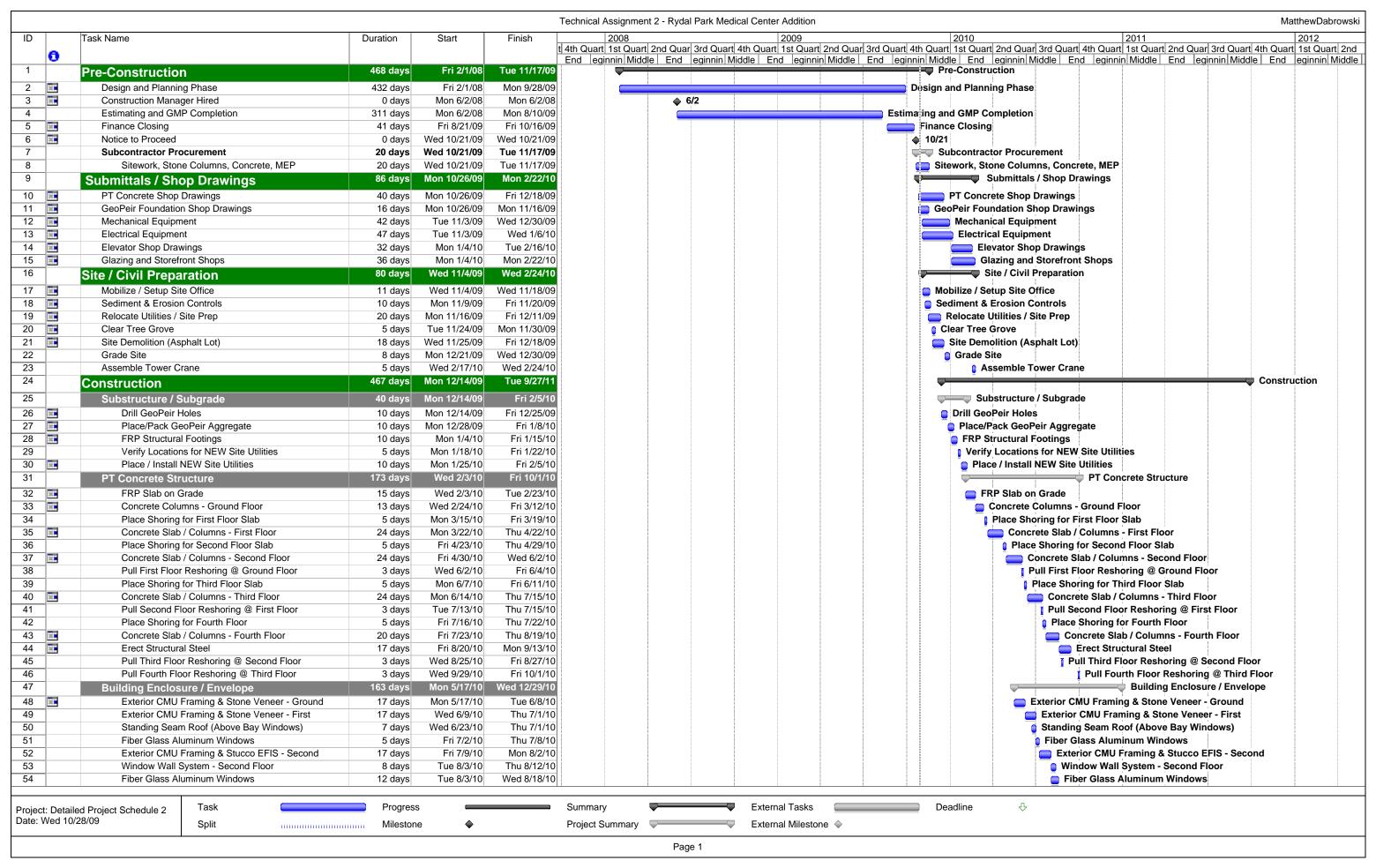
After observing and absorbing each industry member's comments, several topics resonated for potential thesis research during next semester. First, the fact that integrated project delivery has such a weak definition must be addressed. With the technology and resources available in today's world, it is unacceptable that this industry doesn't implement proven collaboration techniques more often. This is definitely a major issue with the project team for Rydal Park. The project team is disconnected given that the owner is from Pennsylvania, the developer is from Texas, the architect is from North Carolina and the interior designer is from Tennessee. Adding to this geographical difficulty, each entity believes that their agenda superseded that of the others.

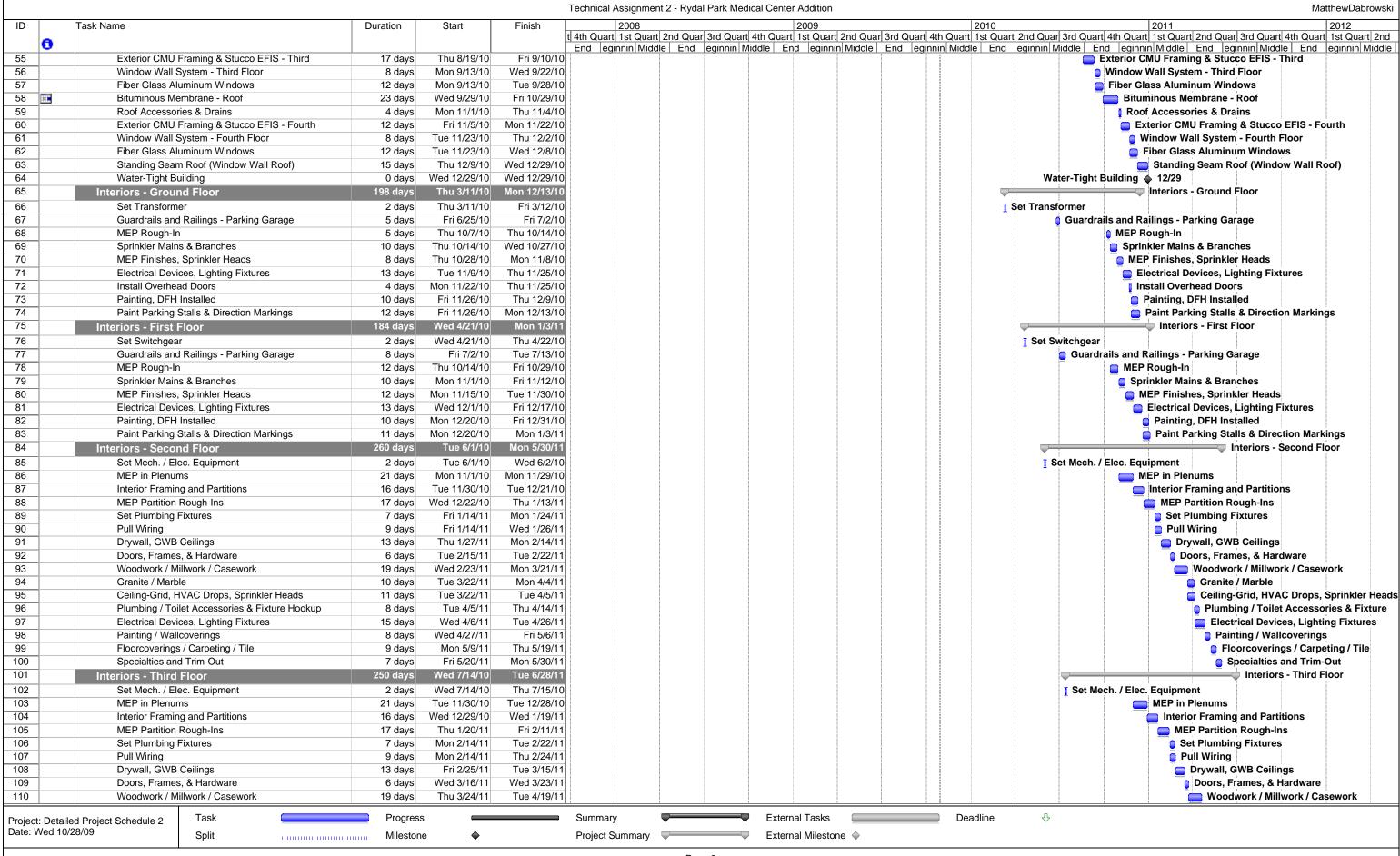
Another striking issue was the use of joint ventures not necessarily as GC-GC but GC-Architect. If general contractors and architects started teaming up more often, they could affect the level of competition and force the industry to utilize collaboration and integration.

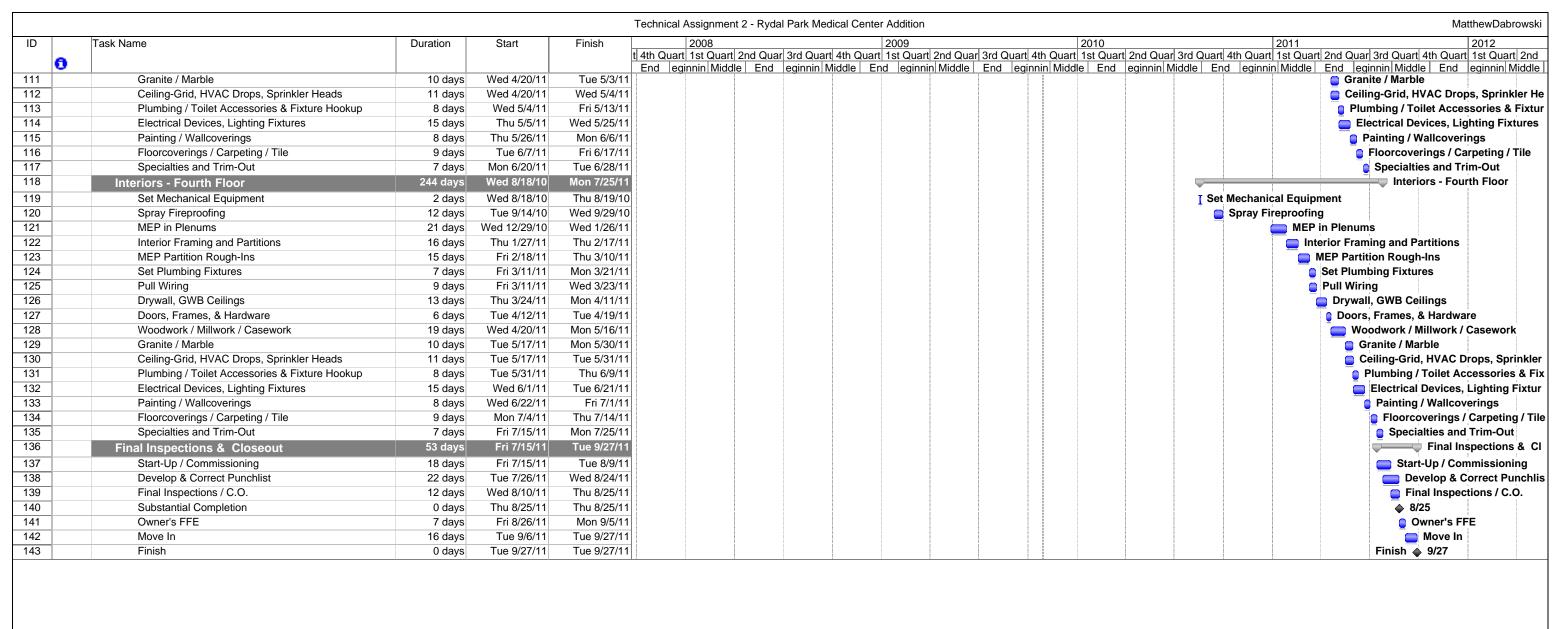
Topics from the BIM and energy sessions could also pertain to future research but have limited relevance to this Medical Center. Both LEED and BIM have not been implemented within the design process, but considering the project's young stage, performing the right form of analysis could have a huge impact on the final delivery process.

[APPENDIX A]

Detailed Project Schedule







Project: Detailed Project Schedule 2 Date: Wed 10/28/09

Task
Split

Progress
Summary
Froject Summary
Froject Summary
Froject Summary
Froject Summary
Fixernal Tasks
Deadline
External Milestone

[APPENDIX B]

Construction Phase Specific Site Plans

RETIREMENT **ADDITION** CENTER CARE CONTINUING MEDICAL PARK COMMUNITY RYDAL

EXCAVATION PLAN

SPECIFIC CONSTRUCTION PHASE:

PENNSYLVANIA

RYDAL,

DRAWN BY:
MATT DABROWSKI

DATE: 10/28/2009



RETIREMENT **ADDITION** CENTER CARE CONTINUING MEDICAL PARK COMMUNITY RYDAL

PENNSYLVANIA

RYDAL,

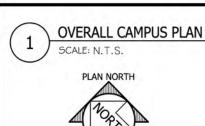
ERECTION PLAN

PHASE:

SPECIFIC CONSTRUCTION

DRAWN BY:
MATT DABROWSKI

DATE: 10/28/2009



RETIREMENT **ADDITION** CENTER CARE CONTINUING MEDICAL PARK COMMUNITY RYDAL

PLAN

INTERIORS

PHASE:

CONSTRUCTION

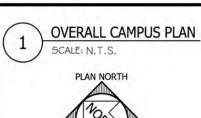
SPECIFIC

PENNSYLVANIA

RYDAL

DRAWN BY:
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[APPENDIX C]

Detailed Post Tension Concrete Structural Estimate

[Submitted: 10/28/2009]

						Foo	ting (Cor	ncrete an	d Rebar)	Take-off					
Mark	Quantity	Depth (ft)	Siz	e (f	ft)		Top Rei	nforcing		Bot. Reinforcing				CY per EA	Total CY
						Short	Long	Bar#	lbs.	Short	Long	Bar#	lbs.		
F1	13	1.00	5.0	х	5.0	N/A		N/A		8	8	5	83	0.93	12.04
F2	5	1.00	6.0	х	6.0	N/A		N/A		7	7	5	88	1.33	6.67
F3	6	1.00	7.0	х	7.0	N/A		N/A		7	7	6	147	1.81	10.89
F4	8	1.33	8.0	х	8.0	N/A		N/A		8	8	6	192	3.16	25.28
F5	7	1.67	9.0	х	9.0	N/A		N/A		9	9	6	243	5.00	35.00
F6	2	1.67	10.0	х	10.0	N/A		N/A		10	10	7	409	6.17	12.35
F7	10	2.00	11.0	х	11.0	N/A	***************************************	N/A		11	11	7	495	8.96	89.63
F8	1	2.00	12.0	х	12.0	N/A		N/A		14	14	7	687	10.67	10.67
F9	8	2.33	13.0	х	13.0	N/A		N/A		11	11	8	764	14.60	116.84
F10	3	2.33	14.0	х	14.0	N/A		N/A		13	13	8	972	16.94	50.81
F11	2	2.67	15.0	х	15.0	N/A		N/A		15	15	8	1202	22.22	44.44
F12	4	2.67	16.0	х	16.0	N/A		N/A		17	17	8	1452	25.28	101.14
F13	1	3.00	17.0	х	17.0	N/A		N/A		18	18	8	1634	32.11	32.11
F14	1	3.00	19.0	х	19.0	N/A		N/A		17	17	9	2196	40.11	40.11
F15 - 1	1	2.67	20.0	x	29.0	24	35	8	3976	40	29	8	4366	57.28	57.28
F15 - 2	1	2.67	15.0	x	30.7	18	37	8	3744	52	22	8	3838	45.51	45.51
F15 - 3	1	2.67	22.0	x	26.8	26	32	8	3858	76	32	8	6715	58.30	58.30
F16 - 1	1	3.00	22.0	x	38.3	26	46	8	6259	26	32	8	4793	93.70	93.70
F16 - 2	1	3.00	16.0	x .	52.6	19	63	8	9679	19	23	8	4055	93.48	93.48
F16 - 3	1	3.00	32.1	x	36.7	39	44	8	7610	39	46	8	7828	130.79	130.79
F16 - 4	1	3.00	31.0	x ·	42.9	37	52	8	8980	37	45	8	8194	147.82	147.82
F16 - 5	1	3.00	15.0	x	30.5	18	37	8	3701	18	22	8	2480	50.83	50.83
F16 - 6	1	3.00	30.5	x ·	42.0	37	50	8	8632	37	44	8	7906	142.33	142.33
	80	Footers							56440	lbs.			60740	lbs.	1408

Footing Formwork Takeoff

Mark	Quantity	Depth (ft)		Size (ft)		Totals
F1	13	1.00	5.0	Х	5.0	140.00
F2	5	1.00	6.0	х	6.0	72.00
F3	6	1.00	7.0	х	7.0	98.00
F4	8	1.33	8.0	х	8.0	192.00
F5	7	1.67	9.0	х	9.0	240.00
F6	2	1.67	10.0	х	10.0	100.00
F7	10	2.00	11.0	х	11.0	484.00
F8	1	2.00	12.0	х	12.0	96.00
F9	8	2.33	13.0	х	13.0	546.00
F10	3	2.33	14.0	Х	14.0	261.33
F11	2	2.67	15.0	х	15.0	240.00
F12	4	2.67	16.0	х	16.0	426.67
F13	1	3.00	17.0	Х	17.0	204.00
F14	1	3.00	19.0	х	19.0	228.00
F15 - 1	1	2.67	20.0	х	29.0	261.33
F15 - 2	1	2.67	15.0	Х	30.7	243.83
F15 - 3	1	2.67	22.0	х	26.8	260.44
F16 - 1	1	3.00	22.0	Х	38.3	362.00
F16 - 2	1	3.00	16.0	х	52.6	411.50
F16 - 3	1	3.00	32.1	х	36.7	412.62
F16 - 4	1	3.00	31.0	х	42.9	443.50
F16 - 5	1	3.00	15.0	х	30.5	273.00
F16 - 6	1	3.00	30.5	х	42.0	435.00

6431.23 SFCA

Slab on G	irade Take Off (Concrete) - Gro	und Floor
Mark	Area (sf)	Thickness (in)	CY
SOG1	2936.8	6	54.4
SOG2	1296.4	4	16.0
SOG3	22258.1	6	412.2
SOG4	802.7	4	9.9
SOG5	2991.1	8	73.9
	30285.1	·	566

	ormwork	
Mark	Perimeter (sf)	
SOG1	108.4	
SOG2	48.0	
SOG3	298.4	
SOG4	37.8	
SOG5	145.8	
	638.4	SFC
		•

1	1st Floor - Concrete					
Area (sf)	Thickness (in.)	CY				
964.2	8	23.8				
2593	10	80.0				
2578.1	8	63.7				
18862.71	9.5	553.1				
1719.16	10	53.1				
3386.54	12.5	130.7				
1055.22	9.5	30.9				
1046.55	9.5	30.7				
32205.48		965.9				

[Submitted: 10/28/2009]

2-	4 Floors - Concre	ete
Area (sf)	Thickness (in.)	CY
964.2	8	23.8
2593	10	80.0
2578.1	8	63.7
18862.71	8	465.7
1719.16	10	53.1
3386.54	11	115.0
1055.22	8	26.1
1046.55	8	25.8
32205.48		853.2

1-4 Floors - Formwork									
Area (sf)	Thickness (in.)	Perimeter	SFCA						
964.2	8	124.2	1047.00						
2593	10	203.7	2762.74						
2578.1	. 8	203.1	2713.50						
18862.71	9.5	549.4	19297.63						
1719.16	10	165.9	1857.37						
3386.54	12.5	232.8	3629.01						
1055.22	9.5	129.9	1158.09						
1046.55	9.5	129.4	1148.99						
32205.48	1		33614.33						

Floors 2-4: 25

2559.52 CY

Total SFCA Formwork:

134457 SFCA

Total Concrete in Slabs:

Total Steel in Slabs:

158.64 Tons

3525.4 CY

Reinforcing Ratio - 800 SF - 19.75 CY Concrete

Designation	Bar Size	Length (ft)	Distance (ft)	Spacing (in.)	Quantity	Tons	Volume
H (Hook)	4	9.333	14.5	12	15	0.04676	0.19444
Tendon	N/A	27.6	N/A		3	0.02766	0.11500
Tendon	N/A	29	N/A		2	0.01937	0.08056
10T	5	5.333	29	6	10	0.02781	0.11481
Bx21'@24"	4	21	27.6	24	14	0.09820	0.40833
10T	5	5.666	27.6	6	10	0.02955	0.12198
10T	5	9	29	6	10	0.04694	0.19375
B@24"	4	18	29	24	15	0.09018	0.37500

Steel : Concrete Ratio 0.01972 ~ 2% Total Steel in Slabs: 158.64 Tons

					Grade B	eams					
Mark	Area (cf)	Denth (ft)	Volume (ft ³)	Re	bar - Parall	el with Be	am	Rebar	- Perpend	icular with	Beam
IVIALK	Alea (SI)	Deptii (it)	volume (ft)	Quantity	Bar#	Length	Lbs.	Quantity	Bar#	Width	Lbs.
GB-1	258.93	1.00	258.93	5	5	90.50	471.96	181	5	3.00	566.35
GB-2	242.73	1.00	242.73	3	5	74.25	232.33				
GB-3	26.02	1.00	26.02	3	5	10.75	33.64	8	5	2.33	19.62
GB-4	30.75	1.00	30.75	3	5	13.18	41.24	10	5	2.33	24.05
GB-5	55.80	1.67	93.00	12	6	9.30	167.62	28	6	6.00	251.43
GB-6	82.33	1.17	96.06	5	5	19.00	99.09	38	5	4.33	171.75
GB-7	692.16	1.17	807.52	7	6	98.88	1039.62	198	6	7.00	2079.25
GB-8	100.64	1.00	100.64	2	5	50.32	104.97	50	5	2.00	104.97
GB-9	26.67	1.00	26.67	2	5	13.33	27.81	13	5	2.00	27.81
GB-10	34.41	1.00	34.41	3	5	14.75	46.15	11	5	2.33	26.92
GB-11	54.67	1.17	63.78	6	5	10.25	64.14	10	5	5.33	57.02
GB-12	28.44	1.00	28.44	2	5	21.33	44.50	21	5	1.33	29.67
GB-13	109.07	1.00	109.07	3	5	46.75	146.28	35	5	2.33	85.32
GB-14	31.50	1.00	31.50	2	5	15.75	32.85	12	5	2.00	24.64
GB-15	121.64	1.00	121.64	2	5	30.41	63.44	30	5	4.00	126.87
GB-16	48.00	1.00	48.00	3	6	16.00	72.10	8	5	3.00	25.03
GB-17	62.25	1.00	62.25	3	6	20.75	93.50	10	5	3.00	32.46
GB-18	150.80	1.00	150.80	2	5	75.40	157.28	57	5	2.00	117.96
GB-19	153.39	1.00	153.39	3	5	65.75	205.73	49	5	2.33	119.99
GB-20	242.67	1.17	283.11	6	5	45.50	284.74	91	5	5.33	506.20
GB-21	736.00	1.17	858.66	6	5	138.00	863.60	276	5	5.33	1535.29
GB-22	120.86	1.00	120.86	2	5	60.43	126.06	60	5	2.00	126.06
GB-23	45.00	1.00	45.00	5	5	15.00	78.23	30	5	3.00	93.87
GB-24	76.66	1.00	76.66	2	5	57.50	119.95	43	5	1.33	59.97
	Total	Concrete:	143.33	CY			4616.83	Ibs			6212.50

Formwork

181.00 148.50 21.50 26.36 31.00 44.33 230.72 100.64 26.67 29.50 23.92 42.67 93.50 31.50 60.82 32.00 41.50 150.80 131.50 106.17 322.00 120.86 30.00 115.00

2142.45 SFCA

5.4 Tons Total Steel:

					Grade	Beams							
Mark	Donth (ft)	Volume (ft ³)	Rebar - Parallel with Beam					Rebar - Perpendicular with Beam					
IVIdIK	Depth (1t)	volume (ft ⁻)	Quantity	Bar#	Length	Lbs.	Quantity	Bar#	Bar Length	Beam Width	Lbs.		
B-1	2.67	113.75	8	8	32.00	683.52	32	3	7.99934	1.33	96.25		
B-2	4.00	168.47	8	9	31.67	861.33	32	4	10.66	1.33	222.79		
B-3	2.67	113.78	8	6	32.00	384.00	32	3	8.00006	1.33	96.26		
B-4	2.67	113.78	8	6	32.00	384.00	32	3	8.00006	1.33	96.26		
B-5	5.00	232.50	15	10	31.00	990.30	31	4	13	1.50	265.98		
B-6	5.00	232.50	15	10	31.00	990.30	31	4	13	1.50	265.98		
B-7	2.50	37.50	4	8	15.00	160.20	15	3	9.5	1.00	53.58		
					***************************************		***************************************				***************************************		
B-8	2.67	113.78	8	6	32.00	384.00	32	3	8.00006	1.33	96.26		
B-9	2.67	113.78	8	6	32.00	384.00	32	3	8.00006	1.33	96.26		
B-10	2.67	113.78	8	6	32.00	384.00	32	3	8.00006	1.33	96.26		
B-11	2.67	113.78	8	6	32.00	384.00	32	3	8.00006	1.33	96.26		
B-12	2.67	113.78	8	6	32.00	384.00	32	3	8.00006	1.33	96.26		
B-13	2.67	113.78	8	6	32.00	384.00	32	3	8.00006	1.33	96.26		
		62.78	CY			6757.64	lbs				1674.64		

ormwork

213.32 295.45 213.34 213.34 356.50 356.50 90.00 213.34 213.34 213.34 213.34 213.34 213.34 3018.46 SFCA

4.2 Tons

[Submitted: 10/28/2009]

Column Concrete and Formwork

Mark	Quantity	First Floor	Second Floor	Third Floor	Fourth Floor	Roof	CV por EA	Total CV		Form	work
IVIAIR	Quantity	Size (in)	Size (in)	Size (in)	Size (in)	Size (in)	CY per EA	TOTAL CY		EA	Total
C2	1	12 x 24					0.99	0.99		79.80	79.80
C3	30	12 x 24	12 x 24	12 x 24	12 x 24		3.55	106.53	ľ	287.60	8628.12
C4	6	16 x 24	16 x 24	12 x 24	12 x 24		4.21	25.25	ľ	305.34	1832.02
C5	4	16 x 24	16 x 24	12 x 24	12 x 24		4.21	16.83	ľ	305.34	1221.35
C6	1	18 x 24	18 x 24	12 x 24	12 x 24		4.54	4.54	ľ	314.20	314.20
C7	0	18 x 24	18 x 24	12 x 24	12 x 24	12 x 24	5.22	0.00	ľ	369.45	0.00
C8	2	20 x 24	20 x 24	12 x 24	12 x 24		4.86	9.73	ľ	323.07	646.14
C9	0	16 x 24	16 x 24	12 x 24	12 x 24		4.21	0.00	ľ	305.34	0.00
C10	13	12 x 24	12 x 24	12 x 24	12 x 24		3.55	46.16	Ĩ	287.60	3738.85
C11	3	12 x 24	12 x 24	12 x 24	12 x 24	12 x 24	4.23	12.70	Ī	342.85	1028.56
C12	3	12 x 24	12 x 24	12 x 24	12 x 24	12 x 24	4.23	12.70	ľ	342.85	1028.56
C13	3	16 x 24	16 x 24	12 x 24	12 x 24		4.21	12.62	ľ	305.34	916.01
C14	1	14 x 24	14 x 24	14 x 24	14 x 24		4.14	4.14	Ī	303.58	303.58
C15	6	16 x 24	16 x 24	16 x 24	16 x 24	12 x 24	5.42	32.50	Ĩ	374.81	2248.86
C16	2	16 x 24	16 x 24	16 x 24	16 x 24		4.73	9.47	Ĩ	319.56	639.12
C17	1	16 x 24	16 x 24	12 x 24	12 x 24		4.21	4.21	ſ	305.34	305.34
C18	2	16 x 28					1.53	3.07	Ī	97.53	195.07
C19	2	12 x 53	12 x 24	12 x 24	12 x 24		4.74	9.48	ľ	351.89	703.77
C20	8	16 x 24	16 x 24				2.63	21.02		177.33	1418.67
C21	1	12 x 27	12 x 27	12 x 27	12 x 27	12 x 27	4.76	4.76		371.42	371.42
	89							337 (CY		25619

Column Rebar – Ground Floor

Mark	Quantity	Grou	ınd I	loor		Vert Rei	nf	Horizontal Reinforcing			
IVIAIK	Quantity	Si	ze (i	n)	Quan	Bar Size	Lbs.	Spacing	Tie Size	Quan	Lbs.
C2	1	12	Х	24	6	9	288.5	12	3	14	31.58
C3	30	12	х	24	4	8	4530.5	12	3	14	947.5
C4	6	16	Х	24	4	9	1153.8	12	3	14	210.6
C5	4	16	х	24	8	10	1947.0	12	3	14	140.4
C6	1	18	х	24	10	10	608.4	12	3	14	36.85
C7	0	18	х	24	8	8		12	3	14	
C8	2	20	Х	24	12	10	1460.3	12	4	14	77.21
C9	0	16	Х	24	10	10		12	3	14	
C10	13	12	х	24	6	9	3749.9	12	3	14	410.6
C11	3	12	Х	24	4	8	453.0	12	3	14	94.75
C12	3	12	Х	24	6	9	865.4	12	3	14	94.75
C13	3	16	Х	24	8	10	1460.3	12	3	14	105.3
C14	1	14	Х	24	8	8	302.0	12	3	14	33.34
C15	6	16	Х	24	6	10	2190.4	9	3	14	210.6
C16	2	16	Х	24	10	10	1216.9	9	3	14	70.19
C17	1	16	Х	24	6	8	226.5	12	3	14	35.09
C18	2	16	Х	28							
C19	2	12	х	53	8	8	604.1	12	3	14	114.1
C20	8	16	х	24	4	5	471.9	16	3	14	280.7
C21	1	12	Х	27	4	8	151.0	12	3	14	34.22

[Submitted: 10/28/2009]

Totals
320.0
5478.0
1364.4
2087.4
645.3
1537.5
4160.5
547.8
960.1
1565.5
335.4
2401.0
1287.1
261.6
718.1
752.7
185.2

12.3 Tons

Column Rebar – First Floor

Mark	Quantity	Fir	rst Flo	or		Vert Rei	nf	Horizontal Reinforcing				
IVIAIK	Quantity	Size (in)			Quan	Bar Size	Lbs.	Spacing	Tie Size	Quan	Lbs.	
C2	1											
C3	30	12	Х	24	4	8	4485.6	12	3	14	947.5	
C4	6	16	х	24	4	9	1142.4	12	3	14	210.6	
C5	4	16	Х	24	8	10	1927.7	12	3	14	140.4	
C6	1	18	Х	24	10	10	602.4	12	3	14	36.85	
C7	0	18	Х	24	8	8		12	3	14		
C8	2	20	Х	24	12	10	1445.8	12	4	14	77.21	
C9	0	16	Х	24	10	10		12	3	14		
C10	13	12	Х	24	6	9	3712.8	12	3	14	410.6	
C11	3	12	Х	24	4	8	448.6	12	3	14	94.75	
C12	3	12	Х	24	6	9	856.8	12	3	14	94.75	
C13	3	16	Х	24	8	10	1445.8	12	3	14	105.3	
C14	1	14	Х	24	8	8	299.0	12	3	14	33.34	
C15	6	16	Х	24	6	10	2168.7	9	3	14	210.6	
C16	2	16	Х	24	10	10	1204.8	9	3	14	70.19	
C17	1	16	Х	24	6	8	224.3	12	3	14	35.09	
C18	2											
C19	2	12	Х	24	4	8	299.0	12	3	14	63.17	
C20	8	16	Х	24	4	5	467.3	16	3	14	280.7	
C21	1	12	Х	27	4	8	149.5	12	3	14	34.22	

Totals
5433.1
1353.0
2068.1
639.3
1523.0
4123.4
543.3
951.6
1551.1
332.4
2379.3
1275.0
259.4
362.2
748.0

11.9 Tons

183.7

Column Rebar - Second Floor

Mark	Quantity	Seco	nd F	loor		Vert Rein	nf	Horizontal Reinforcing			
IVIAIK	Quantity	Size (in)			Quan	Bar Size	Lbs.	Spacing	Tie Size	Quan	Lbs.
C2	1										
C3	30	12	х	24	4	8	3630.1	12	3	11	744.5
C4	6	12	х	24	4	8	726.0	12	3	11	148.9
C5	4	12	х	24	4	8	484.0	12	3	11	99.26
C6	1	12	х	24	10	8	302.5	12	3	11	24.82
C7	0	12	х	24	6	9		12	3	11	
C8	2	12	х	24	6	9	462.3	12	4	11	49.63
C9	0	12	х	24	6	9		12	3	11	
C10	13	12	х	24	6	9	3004.7	12	3	11	322.6
C11	3	12	х	24	4	8	363.0	12	3	11	74.45
C12	3	12	х	24	6	9	693.4	12	3	11	74.45
C13	3	12	х	24	6	9	693.4	12	3	11	74.45
C14	1	14	х	24	8	8	242.0	12	3	11	26.19
C15	6	16	х	24	6	10	1755.1	9	3	11	165.4
C16	2	16	х	24	10	8	605.0	9	3	11	55.15
C17	1	12	х	24	6	8	181.5	12	3	11	24.82
C18	2										
C19	2	12	х	24	4	8	242.0	12	3	11	49.63
C20	8										0
C21	1	12	х	27	4	8	121.0	12	3	11	26.88

[Submitted: 10/28/2009]

Totals
4374.6
874.9
583.3
327.3
511.9
3327.3
437.5
767.8
767.8
268.2
1920.5
660.2
206.3
291.6
147.9
7.7

7.7 Tons

Column Rebar – Third Floor

Mark	Quantity	Thi	rd Fl	oor		Vert Reir	nf	Horizontal Reinforcing			
IVIAIK	Quantity	Si	ze (i	n)	Quan	Bar Size	Lbs.	Spaci	ng Tie Size	Quan	Lbs.
C2	1										
C3	30	12	х	24	4	8	3630.1	12	3	11	744.5
C4	6	12	х	24	4	8	726.0	12	3	11	148.9
C5	4	12	х	24	4	8	484.0	12	3	11	99.26
C6	1	12	х	24	10	8	302.5	12	3	11	24.82
C7	0	12	х	24	6	9		12	3	11	
C8	2	12	х	24	6	9	462.3	12	4	11	49.63
C9	0	12	х	24	6	9		12	3	11	
C10	13	12	х	24	6	9	3004.7	12	3	11	322.6
C11	3	12	х	24	4	8	363.0	12	3	11	74.45
C12	3	12	х	24	6	9	693.4	12	3	11	74.45
C13	3	12	х	24	6	9	693.4	12	3	11	74.45
C14	1	14	х	24	8	8	242.0	12	3	11	26.19
C15	6	16	х	24	6	10	1755.1	9	3	11	165.4
C16	2	16	х	24	10	8	605.0	9	3	11	55.15
C17	1	12	х	24	6	8	181.5	12	3	11	24.82
C18	2										
C19	2	12	Х	24	4	8	242.0	12	3	11	49.63
C20	8										0
C21	1	12	Х	27	4	8	121.0	12	3	11	26.88

Totals
4374.6
874.9
583.3
327.3
511.9
3327.3
437.5
767.8
767.8
268.2
1920.5
660.2
206.3
291.6
147.9

7.7 Tons

Column Rebar – Fourth Floor

Mark	Quantity	Fourth Floor				Vert Rei	nf	Horizontal Reinforcing					
IVIAIR	Quantity	Si	ze (i	n)	Quar	Bar Size	Lbs.	Spacin	g Tie Siz	e Quan	Lbs.		
C2	1												
C3	30												
C4	6												
C5	4												
C6	1												
C7	0	12	Х	24	6	9	231.1	12	3	11	41.36		
C8	2												
C9	0												
C10	13												
C11	3	12	Х	24	4	8	363.0	12	3	11	24.82		
C12	3	12	Х	24	6	9	693.4	12	3	11	28.95		
C13	3												
C14	1												
C15	6	12	Х	24	6	10	1755.1	9	3	11	28.95		
C16	2												
C17	1												
C18	2												
C19	2												
C20	8												
C21	1	12	Х	27	4	8	121.0	12	3	11	26.88		

387.8 722.3 1784.1

1.5 Tons

Total in all Floors: 41.16 Tons

[Submitted: 10/28/2009]

	Shear Wall 1 (20' - 6" Wide)														
20.5	Co	oncrete		Steel (Each Face)						Miscellaneous - Opening Steel					
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	bs. Horizontal lbs.		lbs.	Vertical		lbs.	Horizontal II		lbs.
Ground	14	14.1354	12.52	62	10	3741	38	6	1161	8	10	486.6	8	10	705.69
First	14	14	12.40	62	10	3705	37	6	1150	8	10	481.94	8	10	705.69
Second	10	11.333	7.17	62	8	1861	23	5	485						
Thrid	10	11.333	7.17	62	8	1861	23	5	485						
Fourth	10	11.333	7.17	62	8	1861	23	5	485						
46.43						13030			3764			968.53			1411 4

F	ormwork
	546.57
	541.33
	445.76
	445.76
	445 76

	Shear Wall 2 (14' - 6" Wide)														
14.5	Co	St	eel (Ea	ach Fac	ce)		Miscellaneous - Opening Steel								
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	lbs. Horizontal I		lbs.	Vertical		lbs.	Horizontal I		lbs.
Ground	14	14.1354	8.86	44	10	2646	38	6	821	8	10	486.6	8	10	499.15
First	14	14	8.77	44	10	2621	37	6	813	8	10	481.94	8	10	499.15
Second	10	11.333	5.07	44	8	1316	23	5	343	8	6	136.18	8	6	174.23
Thrid	10	11.333	5.07	44	8	1316	23	5	343	8	6	136.18	8	6	174.23
Fourth	10	11.333	5.07	44	8	1316	23	5	343	8	6	136.18	8	6	174.23
	32.84				9216			2662			1377.1			1521	

Formwork
376.94
373.33
309.77
309.77
309.77

	Shear Wall 3 (22' - 6" Wide)																		
22.5	Co	Concrete					Steel (Each Face)						Miscellaneous - Opening Steel						
	Thickness (in) Height (ft) CY			Ver	tical	lbs.	lbs. Horizontal		lbs.	Ver	Vertical		Horizontal		lbs.				
Ground	10	14.1354	9.82	45	5	663	28	5	663	8	ļ	117.95	8	5	187.74				
First																			
Second																			
Thrid																			
Fourth																			
	9.82				·	663			663	117.95				187.74					

Formwork
612.53
0.00
0.00
0.00
0.00

	Shear Wall 4 (22' - 6" Wide)															
22.5	Concrete					(Each	Face)			Miscellaneous - Opening Steel						
	Thickness (in)	Height (ft)	CY	Ver	ertical lbs. Horizontal				lbs.	Vertical			H	Horizontal		
Ground	10	14.1354	9.82	68	6	1433	28	5	663	8	5	117.95	8	5	187.74	
First	10	14	9.72	68	6	1420	28	5	657	8	5	116.82	8	5	187.74	
Second	10	11.333	7.87	68	5	798	23	5	532							
Thrid	10	11.333	7.87	68	5	798	23	5	532							
Fourth	10	11.333	4.02	35	5	408	23	5	532							
		39.30			4857			2916			234 76			375 48		

Formwo	rk
612. 606. 491. 491.	67 10 10

	Shear Wall 5 (19' - 0" Wide)														
19.0	Co	ncrete			Steel	(Each	ach Face)			Miscellaneous - Openings					
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs. Horizontal		lbs.	Vertical		lbs.	Horizontal		al	
Ground	10	14.1354	8.29	57	6	1210	38	5	747						
First	10	14	8.21	57	6	1199	37	5	740						
Second	10	11.333	6.65	57	5	674	23	5	449						
Thrid	10	11.333	6.65	57	5	674	23	5	449						
Fourth	10	11.333	6.65	57	5	674	23	5	449						
			36.44		·	4431		·	2834						

ormwork
513.59
508.67
411.77
411.77
411.77

	Shear Wall 6 (22' - 4" Wide)														
22.3	Co		Rebar (Each Face)						Miscellaneous - Openings						
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs. Horizontal			lbs.	Vertical			Horizontal		
Ground	10	14.1354	9.74	45	5	659	28	5	659						
First	10	14	9.65	45	5	652	28	5	652						
Second	10	11.333	7.81	45	5	528	23	5	528						
Thrid	10	11.333	7.81	45	5	528	23	5	528						
Fourth	10	11.333	7.81	45	5	528	23	5	528						
			42.83			2895			2895						

Formwork
607.82
602.00
487.32
487.32
487.32

					Shea	r Wall 7	' (10' -	10" W	ide)						
10.8	3 Concrete Rebar									Miscellaneous - Openings					
	Thickness (in)	Height (ft)	CY	Vertical lbs. Horizontal lbs. Vertical						H	Horizontal				
Ground	10	14.14	4.73	22	5	319	28	5	319						
First	10	14.00	4.68	22	5	316	28	5	316						
Second	10	11.33	3.79	22	5	256	23	5	256						
Thrid	10	11.33	3.79	22	5	256	23	5	256						
Fourth	10	11.33	3.79	22	5	256	23	5	256						
			20.78			1404			1404						

ormwork
282.71
280.00
226.66
226.66
226.66

					She	ar Wall	8 (14' -	6" Wi	de)						
14.5	Concrete Rebar									Miscellaneous - Openings					
	Thickness (in)	Height (ft)	CY	Ver	ertical Ibs. Horizontal Ibs. Vertical Hori						Horizonta	rizontal			
Ground	10	14.14	6.33	29	5	428	28	5	428						
First	10	14.00	6.27	29	5	423	28	5	423						
Second	10	11.33	5.07	29	5	343	23	5	343						
Thrid	10	11.33	5.07	29	5	343	23	5	343						
Fourth	10	11.33	5.07	29	5	343	23	5	343						
			27.81			1879			1879						

Formwork
206.27
386.37
382.67
309.77
309.77
309.77

Total Concrete	256	CY

38375 lbs. 19019 lbs. 2698.3 lbs.

3495.6 lbs. 15280.15 SFCA

Total Steel: 31.794 Tons

[Submitted: 10/28/2009]

				Elevat	or Tow	er Sout	n Wall	(16' - 8	" Wide)											
16.67	Co	Concrete					ach Fac	e)			Misce	llaneous -	Openi	ng Ste	Steel					
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	Horizontal lbs.		Ver	Vertical lbs.		Horizontal Ibs.							
Ground	10	14.1354	7.27	33	5	491	28	5	491	8	5	117.95	8	5	139.07					
First	10	14	7.20	33	5	487	28	5	487	8	5	116.82	8	5	139.07					
Second	10	11.333	5.83	33	5	394	23	5	394	8	5	94.56	8	5	139.07					
Thrid	10	11.333	5.83	33	5	394	23	5	394	8	5	94.56	8	5	139.07					
Fourth	10	11.333	5.83	33	5	394	23	5	394	8	5	94.56	8	5	139.07					
			31.96			2160			2160			518.45			695.33					

Formwork
447.62
443.33
358.88
358.88
358.88

				Elevato	or Tow	er West	Wall (:	11' - 2.	5" Wide)					
11.21	Co	•	Steel (Ea	ach Fac	e)		Miscellaneous - Opening Steel								
	Thickness (in)	Height (ft)	CY	Vertical lbs. Horizontal lbs. Vertical lbs.						lbs.	Horizontal lbs.				
Ground	10	14.1354	4.89	22	5	330	28	5	330						
First	10	14	4.84	22	5	327	28	5	327						
Second	10	11.333	3.92	22	5	265	23	5	265						
Thrid	10	11.333	3.92	22	5	265	23	5	265						
Fourth	10	11.333	3.92	22 5 265 23 5 265											
			21.49			1453			1453						

293.31	
290.50	
235.16	
235.16	

235.16

Formwork

				Elevato	or Tow	er East V	Vall (1	2' - 5.25	5" Wide)					
12.44	Co	Steel (Ea	ach Fac	e)		Miscellaneous - Opening Steel									
	Thickness (in)	Height (ft)	CY	Vertical Ibs. Horizontal Ibs. Vertical Ibs						lbs.	Horizontal lbs.				
Ground	10	14.1354	5.43	25	5	367	28	5	367						
First	10	14	5.37	25	5	363	28	5	363						
Second	10	11.333	4.35	25	5	294	23	5	294						
Thrid	10	11.333	4.35	25	5	294	23	5	294						
Fourth	10	11.333	4.35	25	5	294	23	5	294						
	_		23.85			1612			1612						

Formwork
328.06
324.92
263.02
263.02
263.02

				Elevat	or Tow	er Nortl	n Wall	(16 ['] - 8	" Wide)						
16.67	Co	oncrete			Ş	Steel (Ea	ach Fac	e)			Misce	llaneous -	Openi	ng Ste	el
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	ontal	lbs.	Vert	tical	lbs.	Horiz	ontal	lbs.
Ground	10	14.1354	7.27	33	5	491	28	5	491	16	5	235.89	8	5	139.07
First	10	14	7.20	33	5	487	28	5	487	16	5	233.63	8	5	139.07
Second	10	11.333	5.83	33	5	394	23	5	394	16	5	189.13	8	5	139.07
Thrid	10	11.333	5.83	33	5	394	23	5	394	16	5	189.13	8	5	139.07
Fourth	10	11.333	5.83	33	5	394	23	5	394	16	5	189.13	8	5	139.07
			31.96			2160			2160			1036.90			695.33

Formwork
447.62
443.33
358.88
358.88
358.88

			9	Stair To	wer N	O. 1 Sou	th Wal	l (23' -	0" Wide	·)					
23.00	Co	oncrete			:	Steel (Ea	ach Fac	æ)			Misce	llaneous -	Openir	ng Stee	el
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	ontal	lbs.	Verl	ical	lbs.	Horizo	ontal	lbs.
Ground	12	14.1354	12.04	46	5	678	28	5	678	8	5	117.95	8	5	191.91
First	12	14	11.93	46	5	672	28	5	672	8	5	116.82	8	5	191.91
Second	12	11.333	9.65	46	5	544	23	5	544						
Thrid	12	11.333	9.65	46	5	544	23	5	544						
Fourth	12	11.333	9.65	46	5	544	23	5	544						
			52.93			2981			2981			234.76			383.82

Formwork

621.96 616.00 498.65 498.65 498.65

			9	Stair To	wer N	0.1 We	st Wal	l (11' -	0" Wide)					
11.00	Co	ncrete				Steel (Ea		•	· · · · · · ·		Misce	llaneous -	Openi	ng Ste	el
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	ontal	lbs.	Ver	tical	lbs.	Horiz	ontal	lbs.
Ground	12	14.1354	5.76	22	5	324	28	5	324	8	5	117.95			
First	12	14	5.70	22	5	321	28	5	321	8	5	116.82			
Second	12	11.333	4.62	22	5	260	23	5	260	8	5	94.56	8	5	91.78
Thrid	12	11.333	4.62	22	5	260	23	5	260	8	5	94.56	8	5	91.78
Fourth	12	11.333	4.62	22	5	260	23	5	260	8	5	94.56	8	5	91.78
			25.31			1426			1426			518.45			275.35

Formwork

282.71 280.00 226.66 226.66 226.66

			9	Stair To	wer N	O. 1 Nor	th Wal	l (23' -	0" Wide)					
23.00	Co	oncrete				Steel (Ea	ach Fac	æ)			Misce	llaneous -	- Open	ing Ste	el
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	ontal	lbs.	Ver	tical	lbs.	Horiz	zontal	lbs.
Ground	12	14.1354	12.04	46	5	678	28	5	678						
First	12	14	11.93	46	5	672	28	5	672						
Second	12	11.333	9.65	46	5	544	23	5	544						
Thrid	12	11.333	9.65	46	5	544	23	5	544						
Fourth	12	11.333	9.65	46	5	544	23	5	544						
			52.93			2981			2981						

Formwork

621.96 616.00 498.65 498.65 498.65

				Stair To	ower N	IO. 1 Eas	st Wall	(11' - 0)" Wide						
11.00	Cc	oncrete				Steel (Ea	ach Fac	:e)			Misce	llaneous -	Openi	ng Ste	el
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	ontal	lbs.	Ver	tical	lbs.	Horiz	ontal	lbs.
Ground	12	14.1354	5.76	22	5	324	28	5	324						
First	12	14	5.70	22	5	321	28	5	321						
Second	12	11.333	4.62	22	5	260	23	5	260	8	5	94.56	8	5	91.78
Thrid	12	11.333	4.62	22	5	260	23	5	260	8	5	94.56	8	5	91.78
Fourth	12	11.333	4.62	22	5	260	23	5	260	8	5	94.56	8	5	91.78
			25.31			1426			1426			283.69			275.35

Formwork

282.71 280.00 226.66 226.66 226.66

	Stair Tower NO. 2 West Wall (22' - 0" Wide)														
22.00	Co	ncrete			St	eel (Ea	ach Fac	:e)		1	∕liscell	aneous -	Openi	ng Ste	el
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	ontal	lbs.	Ver	tical	lbs.	Horiz	ontal	lbs.
Ground	12	14.1354	11.52	44	44 5 649 28 5 649 8 5 117.95 8 5 18										
First	12	14	11.41	44	5	28	116.82	8	5	183.57					
Second	12	11.333	9.23	44	5	520	23	5	520						
Thrid	12	11.333	9.23	44	5	520	23	5	520						
Fourth	12	11.333	9.23	44	5	520	23	5	520						
			50.63	50.63 2851 2851 234.76 367.14											

Formwork
593.69
588.00
475.99
475.99
475.99

			Sta	ir Tow	er NO.	2 Nort	th Wall	(10' -	6" Wid	e)					
10.50	Co	ncrete			St	eel (Ea	ach Fac	e)		ı	Miscell	aneous -	Openi	ng Ste	el
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	ontal	lbs.	Ver	tical	lbs.	Horiz	ontal	lbs.
Ground	12	14.1354	5.50	21	5	310	28	5	310						
First	12	14	5.44	21	5	307	28	5	307						
Second	12	11.333	4.41	21	5	248	23	5	248						
Thrid	12	11.333	4.41	21	5	248	23	5	248	8	5	94.56			
Fourth	12	11.333	4.41	21	5	248	23	5	248	8	5	94.56	8	5	87.61
		_	24.16			1361			1361			189.13			87.61

268.57 266.00 215.33 215.33 215.33	Formwork
	266.00 215.33 215.33

			Si	tair Tov	wer NC). 2 Eas	t Wall	(22' - 0	" Wide)					
22.00	Co	ncrete			St	eel (Ea	ach Fac	e)		ľ	∕liscell	aneous -	Openi	ng Ste	el
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	ontal	lbs.	Ver	tical	lbs.	Horiz	ontal	lbs.
Ground	12	14.1354	11.52	44	5	649	28	5	649	8	5	117.95	8	5	183.57
First	12	14	11.41	44	5	642	28	5	642	8	5	116.82			
Second	12	11.333	9.23	44	5	520	23	5	520	8	5	94.56	8	5	183.57
Thrid	12	11.333	9.23	44	5	520	23	5	520	8	5	94.56	8	5	183.57
Fourth	12	11.333	9.23	44	5	520	23	5	520	8	5	94.56			
			50.63			2851			2851			518.45			550.70

Formwork
E03 C0
593.69 588.00
475.99
475.99
475.99

Stair Tower NO. 2 South Wall (10' - 5" Wide)															
10.50	Concrete			Steel (Each Face)					Miscellaneous - Opening Steel						
	Thickness (in)	Height (ft)	CY	Ver	tical	lbs.	Horiz	ontal	lbs.	Vertical lb:		lbs.	Horizontal lbs.		
Ground	12	14.1354	5.50	21	5	310	28	5	310						
First	12	14	5.44	21	5	307	28	5	307						
Second	12	11.333	4.41	21	5	248	23	5	248						
Thrid	12	11.333	4.41	21	5	248	23	5	248						
Fourth	12	11.333	4.41	21	5	248	23	5	248						
			24 16			1361			1361						

Formwork
268.57
266.00
215.33
215.33
215.33

Total Concrete: 416 C

[Submitted: 10/28/2009]

24624 lbs.

24624 lbs.

3535 lbs.

22200 SFCA 3331 lbs.

Total Steel: 28.06 Tons

[APPENDIX D]

General Conditions Estimate

[Submitted: 10/28/2009]

DIVISION. 01 - GENERAL CONDITIONS	MEDIC	AL CENT	ER ADDITIO	ON ESTIMATE
DESCRIPTION	QTY.			TOTAL COSTS
DESCRIPTION.	Q I I.	OIIII	Citii CCC1	101712 00010
MOBILIZATION AND TEMPORARY FIELD OFFICES/EXPENSES				
WT SUPERINTENDENT FIELD OFFICE	1	LS	\$25,000.00	\$25,000
WT FIELD OFFICE CONSTRUCTION / DEMO	1	LS		BY OWNER
TRAILER ELECTRICAL / TELEPHONE CONNECTION	1		\$5,000.00	
TRAILER TELEPHONE SERVICE - FAX LINE	18	MO		BY OWNER
TRAILER TELEPHONE EQUIPMENT	1	LS		BY OWNER
TRAILER ELECTRIC SERVICE	0			BY OWNER
TRAILER WATER / SANITARY CONNECTION / TANK	1	ALLOW		BY OWNER
TRAILER WATER SERVICE	0	_		BY OWNER
TRAILER ACCESS PLATFORMS AND MISC CARPENTRY - SUPPLY AND REMOVE	1	LS	\$2,500.00	
OFFICE FURNITURE	1	LS	\$2,500.00	
OFFICE SUPPLIES	17	MO		
	17	MO	\$150.00	
OFFICE POSTAGE & SHIPPING			\$100.00	
COMPUTER INTERNET SERVICE	18	MO	-	BY OWNER
OFFICE FAX MACHINE	1	EA	\$300.00	
OFFICE PRINTER	2	EA	\$300.00	
COLOR PRINTER	1	EA	\$500.00	
SCANNER	1	EA	\$200.00	· · · · · · · · · · · · · · · · · · ·
OFFICE COPIER (RENT-W/SERVICE AGREEMENT)	18	MO		BY OWNER
PRINTER CONSUMABLES (TONER, PRINTER CARTRIDGES, ETC)	17	MO	\$50.00	
PLOTTER	0			\$0
FILE SERVER	1	EA		BY OWNER
OFFICE TRAILER CLEANING SERVICE	18	MO		BY OWNER
OFFICE TRAILER DUMPSTER/TRASH REMOVAL	18	MO	\$250.00	\$4,500
FIELD OFFICE TRAILER INSURANCE	1.5	YRS	\$250.00	\$375
			SUBTOTAL:	\$45,075
SMALL TOOLS AND EQUIPMENT				
MISCELLANEOUS MILEAGE	17	MO	\$100.00	\$1,700
MISC. SMALL TOOLS-BROOMS, GARBAGE CANS, MOPS ETC	1	LS	\$750.00	\$750
MISC. SUPPLIES	17	MO	\$100.00	\$1,700
			SUBTOTAL:	\$4,150
PROJECT MANAGEMENT AND SUPERVISION				
JESSE BEAM - SENIOR PROJECT MANAGER	1440	HRS	\$120.00	\$172,800
LAWSON KILBOURNE - SUPERINTENDENT	2960	HRS	\$95.00	\$281,200
CHIP CINAMELLA - PROJECT MANAGER	3240	HRS	\$95.00	
BOGDAN MINDA - PROJECT MANAGER	3240	HRS	\$95.00	
SHELLY CHRISTMAN - ASSISTANT PROJECT MANAGER	1800	HRS	\$80.00	\$144,000
KEN FONDE - PROJECT ENGINEER	3240	HRS	\$70.00	\$226,800
FIELD ENGINEER	2500		\$70.00	\$175,000
PROJECT ACCOUNTANT/CLERICAL	2960		\$40.00	
			SUBTOTAL:	\$1,733,800
TRAVEL AND LODGING			SOBIOTAL.	ψ1,135,300
JESSE BEAM - SENIOR PROJECT MANAGER	17	MO	\$400.00	\$6,800
DAILY COMMUTES	17		\$2,000.00	
MISC. MILEAGE / TRAVEL COSTS	17	MO	\$2,000.00	
MIGC. WILLEAGE / TRAVEL COSTS	''	IVIO	SUBTOTAL:	\$45,050
PLANS, PERMITS AND POSTAGE			SOBIOTAL.	\$45,USU
	450	CETC	\$200.00	¢20,000
DRAWINGS AND SPECIFICATIONS BID SETS	150		\$200.00	
DRAWINGS AND SPECIFICATIONS-ROUTINE UPDATES/BULLETINS	17	MO	\$250.00	* ,
BUILDING / SPECIAL PERMITS		N/A	#050	BY OWNER
OVERNIGHT EXPRESS CHARGES / FEDEX / UPS	17	MO	\$250.00	
POSTAGE AND SHIPPING-BID PERIOD	2		\$750.00	* /
SHOP DRAWINGS AND SAMPLES		ALLOW	\$1,000.00	
RED-LINE AS-BUILT DRAWING COPIES	1	ALLOW	\$1,000.00	
		1	SUBTOTAL:	\$42,000

[Submitted: 10/28/2009]

ODEOLAL DECUMPENTA				
SPECIAL REQUIREMENTS				
PROGRESS PHOTOS-MONTHLY UPDATES	17	MO	\$50.00	
FINAL PHOTOS	1	LS	\$2,000.00	* ,
PROGRESS PHOTOS-DIGITAL CAMERA	1	EA	\$400.00	
AERIAL PHOTOS (MONTHLY)	17	MO	\$300.00	. ,
PROGRESS MEETINGS	17	МО	\$100.00	\$1,700
MONTHLY REPORTS	17	MO	\$100.00	\$1,700
CPM SCHEDULE-SET UP / INDEPENDENT CONSULTANT	1	LS	\$7,500.00	\$7,500
CPM SCHEDULE UPDATES	17	MO	\$500.00	\$8,500
ARCHITECT AND ENGINEERING FEES		N/A		BY OWNER
PUNCHLIST/CLOSEOUT	1	ALLOW	\$2,500.00	\$2,500
QUALITY CONTROL PROGRAM	1	EA	\$500.00	\$500
QUALITY CONTROL AWARDS	17	MO	\$50.00	\$850
LOSS PREVENTION PROGRAM	1	EA	\$500.00	\$500
SAFETY PROGRAM	1	EA	\$500.00	\$500
SAFETY AWARDS	17	MO	\$100.00	\$1,700
MISC JOB STORAGE TRAILERS	17	МО	\$500.00	\$8,500
JOB DRINKING WATER	17	MO	\$250.00	\$4,250
			SUBTOTAL:	· · · · · · · · · · · · · · · · · · ·
TESTING & INSPECTIONS				, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
EXTERIOR SKIN WATER/LEAK TEST	5	DAYS	\$2,500.00	\$12,500
INDEPENDENT TESTING & INSPECTION		LS	Ψ2,000.00	BY OWNER
INDET ENDERT TECTING & INCITECTION			SUBTOTAL:	\$12,500
SITE REQUIREMENTS			SOBIOTAL.	\$12,500
TEMPORARY FENCES / PEDESTRIAN PROTECTION (~ 1,000 LF)	1	LS	\$15,000.00	\$15,000
GATES TEMPODARY A COESC BOARD	3	EA	\$750.00	. ,
TEMPORARY ACCESS ROADS	0	LS	\$0.00	
TEMPORARY PARKING / LAYDOWN	1	LS	\$15,000.00	·
MAINTAIN ACCESS ROADS & PARKING	17	MO	\$500.00	. ,
SURVEY AND ESTABLISH BENCHMARKS	1	ALW	\$5,000.00	. ,
SAFETY MAINTENANCE	17	МО	\$1,000.00	
BARRICADES & SAFETY	1	ALW	\$25,000.00	
FLOOR OPENING PROTECTIONS	1	ALW	\$7,500.00	. ,
ELEVATOR SHAFTS OPENING PROTECTION	10	EA	\$500.00	
WEATHER & DUST PROTECTION	1	ALW	\$10,000.00	
TEMPORARY FIRE PROTECTION-EXTINGUISHERS (1 EVERY 3,000 SF)	50	EA	\$75.00	
FLOOR PROTECTION		SF		BY SUB
DAILY CLEANUP - LABORERS	12	МО	\$7,500.00	\$90,000
FINAL CLEANING	90000	SF	\$2.34	\$210,600
FINAL WINDOW CLEANING	1	LS	\$15,000.00	\$15,000
DUMPSTER SERVICE	17	MO	\$5,000.00	\$85,000
STREET CLEANING	12	MO	\$2,000.00	\$24,000
SNOW REMOVAL (INSIDE JOB FENCE ONLY)	1	ALW	\$10,000.00	\$10,000
TEMPORARY SANITARY FACILITIES (PORTA TOILETS)	17	MO	\$1,000.00	\$17,000
TEMPORARY POWER / WATER CONSUMPTION		MO		BY OWNER
SELECT TEMPORARY HEAT	4	MO	\$10,000.00	\$40,000
PROJECT SIGN	2	EA	\$750.00	\$1,500
CONSTRUCTION SIGNAGE	1	LS	\$2,500.00	\$2,500
MAINTAIN SEDIMENT AND EROSION CONTROL	17	MO	\$500.00	\$8,500
STABILIZED CONSTRUCTION ENTRANCE	2	EA	\$5,000.00	\$10,000
CRANE USAGE WITH OPERATORS	1	LS	\$9,520.00	\$9,520
ELEVATOR OPERATOR (BY ELEVATOR CONTRACTOR)	2	MO	\$20,000.00	\$40,000
VENTILATION / NEGATIVE AIR MACHINE - TO BE SUPPLIED AS DICTATED		ALW		BY OWNER
HEPA-VACS / CLEANING SUPPLIES - TO BE SUPPLIED AS DICTATED		ALW		BY OWNER
			SUBTOTAL:	\$631,870
BUILDING ACCESS				
MATERIAL HOIST	6	МО	\$15,000.00	\$90,000
SET-UP / BREAKDOWN OF MATERIAL HOIST	1	LS	\$15,000.00	· · · · · · · · · · · · · · · · · · ·
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TRASH CHUTE		LS	SUBTOTAL:	NOT USED \$105,000

WEEKLY COSTS: \$34,185.83